

Hybrid fiber reinforced self-compacting concrete: hardened properties

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Abstract: In most cases, fiber reinforced self-compacting concrete (FRSCC) contains only one type of fiber. The use of two or more types of fibers in a suitable combination may potentially not only improve the overall properties of self-compacting concrete, but may also result in performance synergie. The combining of fibers, often called hybridization, is investigated in this paper for a cimentitious matrix. Control, single, two fibers hybrid composites were cast using different fiber type steel and polypropylene with different sizes. Flexural toughness tests were performed and results were extensively analysed to identify synergy, if any, associated with various fiber combinations. Based on various analysis schemes, the paper identifies fiber combinations that demonstrate maximum synergy in terms of flexural toughness.

Key words: self-compacting, concrete, hybrid, fibers, toughness, energy

1- Introduction

The concept of the self compacting concrete (SCC), was created and developed at the end of the eighties after the developpement of new organic additives. It has a high flowability and a moderate viscosity, and no blocking may occur during flow. Several mix design methods for SCC were proposed by Okamura and Ouchi [1], Petersson and Billberg [2], and Sedran and De Larrard [3]. The interest of those concretes appeared only at the end of the nineties. Self-compacting concrete is a brittle material with a low strain capacity. Reinforcement of self-compacting concrete with short randomly distributed fibers can adress some of the concerns related to self-compacting concrete brittleness and poor resistance to crack growth. Fibers, used as reinforcement, can be effective in arresting cracks at both micro and macro-levels. At the micro-level, fibers inhibit the initiation and growth of cracks, and after the micro-cracks coalesce into macro-cracks, fibers provide mechanisms that abate their unstable propagation, provide effective bridging, and impart sources of strength gain, toughness and ductility [4,5].

Different types of fibers provide different mechanical behaviours and a controlled mix of them, so called hybrid, are now developped to customise the mechanical response of the material. It is possible to mix fibers of differents sizes, from micro to macro, or of different mechanical properties, or both together.

It has been shown recently [6] that by using the concept of hybridization with two different fibers incorporated in a common cement matrix, the hybrid composite can offer more attractive engineering properties because the presence of one fiber enables the more efficient utilization of the potential properties of the other fiber [7]. However, the hybrid composites studied by previous researchers were focused on cement paste or mortar. The mechanical properties of hybrid fiber reinforced self-compacting concrete at low fiber volume fraction have not been studied previously.

Associations of fibers of differents characteristics have been investigated. Bantia and al [7] showed that the association of synthetic and metal fibers can

Table 1 : properties of the fibers used

| | | | |
|--------------------------------|-------------------------|-------|-------------------------------|
| Fiber type | Amorphous metal | | Polypropylene |
| Length | 20 mm | 30 mm | 50 mm |
| Cross section | Rectangular 1,6*0,03 mm | | Roughly rectangular 1,6*0,4mm |
| Density | 7,2 | | 0,92 |
| Tensile strength | 2000 Mpa | | 310Mpa |
| Elastic modulus | 140Gpa | | 4,3 Gpa |
| Alkali, acid, salts resistance | stainless | | high |

give to the cementitious composite more ductility than what give metal fibers to concrete. Kawamata and al [8] used for self-compacting concrete associations long and short fibers according to these authors, the short fibers prevent the formation of the small cracks, delaying thus the formation of the macro cracks that will be taken by the longest fibers and will need a greater dissipation energy. In this way the ductility of the material will be increased.

However, the fibers are too expensive and the concurrence between the manufacturers is hard : looking for a relation between proportion and fibers number seems to be a solution.

The present work deals with a mix of fibers of similar sizes and different properties and different length. Mixes of fibers of different natures have already been tried, usually to provide or enhance a multicracking behaviour in high performance hybrid fiber reinforced composite [7].

The goal of this study is to reduce the costs of the fibers, in other words, to optimize the quantitative proportions (kg/m^3) of the fibers by taking account of the economic factors: to

reach the desired resistance with the least proportion (lowest cost).

2- Basic choices

2-1 Fibers used

Both are macro fibers, there lengths are respectively 20 and 30 mm for the steel fibers and 50 mm for polypropylene (they exist in those lengths state).

The high bond and high modulus ones are amorphous metal fibers. They are flexible, ribbon like and totally stainless. For this reason they can be used with no reservation in self-compacting concrete.

The low modulus and slipping fibers are polypropylene ones. Designed to be used in replacement of steel fibers, they have similar geometry. They are straight, 50 mm in length, roughly 1.6*1 mm in cross section. They are auto-fibrillating and, although being made of a single piece of polypropylene, they look like a bundle of thinner fibers stuck together. This property provides an improved bond with the cement matrix.

The characteristics of these two fibers types are detailed and compared in Table 1.

Table 2 : self-compacting concrete mix proportions

| Mixes | Composition | | | | | | | | |
|---------|-------------------|-------------------|-------------------|-------------------|-------------------|------------------|----------------------|--------|-------------------|
| | C | F | S | G | SP | E | FM Kg/m ³ | | FS |
| | Kg/m ³ | Kg/m ³ | Kg/m ³ | Kg/m ³ | Kg/m ³ | L/m ³ | l=20mm | L=30mm | Kg/m ³ |
| SCC | 370 | 170 | 820 | 860 | 3.1 | 180 | - | - | - |
| SCCMF1 | 340 | 213 | 884 | 553 | 4.08 | 185 | 10 | - | - |
| SCCMF2 | 340 | 213 | 884 | 553 | 4.08 | 185 | 0 | 6.75 | - |
| SCCSF | 340 | 213 | 884 | 553 | 4.08 | 185 | - | - | 4.5 |
| SCCFMS1 | 340 | 213 | 884 | 553 | 4.08 | 185 | 10 | - | 4.5 |
| SCCFMS2 | 340 | 213 | 884 | 553 | 4.08 | 185 | - | 6.75 | 4.5 |

S : sand 0-4 mm;
 G : gravel 4-10mm ;
 SP : superplasticiser sika3030 ;
 F : filler ; C :cement ;
 FM : metallic fiber ;
 FS : synthetic fiber ;
 E : water.

2-2 Mixes investigated

The target was fiber reinforced self-compacting concrete (SCC). The investigated fiber (l=30mm) content were 10kg/m³ (0.13% by volume) and 6.75 kg/m³ when we use fiber (l=20 mm) for the amorphous metal fibers and 4.5 kg/m³ (0.5 % by volume) for the polypropylene fibers. The hybrid mix was the associate of the contents for the respective fiber types, that is to say 10 kg/m³ of amorphous fibers (l=30 mm) plus 4.5kg/m³ of polypropylene fibers and 6.75kg/m³ of amorphous fibers (l=20mm) plus 4.5 kg/m³ of polypropylene fibers. The behaviour of the matrix itself, with no fibers, was also investigated. The different mixes will be designated as illustrated in the following example : self-compacting concrete with no fibers = SCC, self-compacting concrete with 10 kg/m³ of amorphous metal fibers (l=30mm) = SCCMF1, self-compacting concrete with 6.75 kg/m³ of amorphous fibers (l=20 mm) = SCCMF2, self-compacting concrete with 4.5 kg/m³ of polypropylene fibers = SCCSF, hybrid self-compacting mix with 10kg/m³ of amorphous metal fibers (l=30 mm) plus 4.5

kg/m³ of polypropylene fibers = SCCFMS1, hybrid self-compacting mix with 6.75 kg/m³ of amorphous metal fibers (l=20 mm) plus 4.5 kg/m³ of polypropylene fibers = SCCFMS2.(the mixes are shown in Table 2.)

Six self-compacting reference mixtures with different dosage and type of fiber were developed. Each mix was optimised for its fibre reinforcement. For the self placing mixes, the sand/gravel ratio and the filler content were varied until obtaining the required workability with no segregation [9,10]. The compositions, and the workability parameters of all the mixes are given in Table 2.

For the self-compacting concretes containing hybrid fibers, the dosage of superplasticizer was increased properly to maintain a good workability. The mixtures were batched in 50-l vertical axis concrete mixer. The cement, sand, and fibers were dry-mixed for 30s. This was followed by the addition of coarse aggregate, water, and the superplasticizer, with a mixing time of 5 min. The specimens were demolded after 1 day and then placed in a curing room with 90%

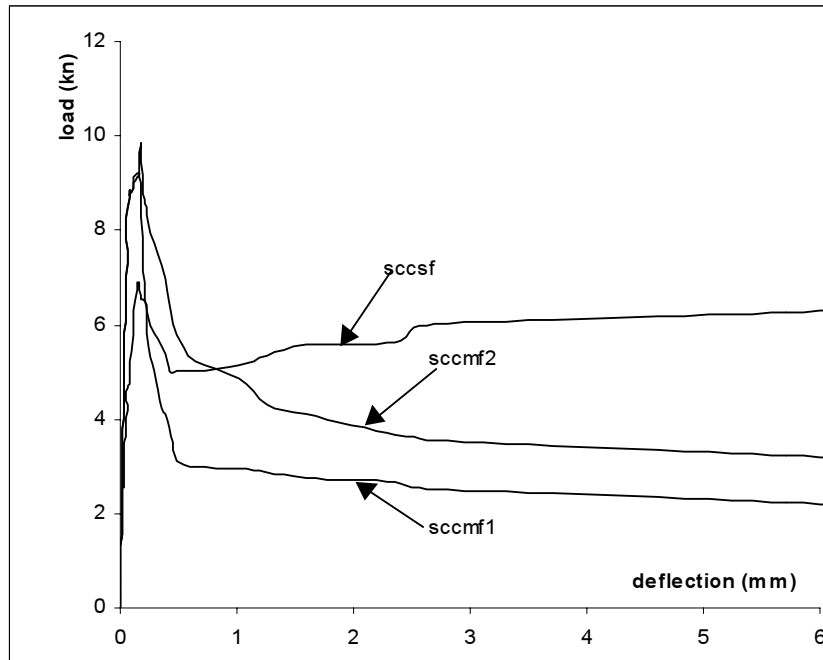


Fig.1: plots of flexural load versus deflection for simple fiber-reinforced self-compacting concrete beams

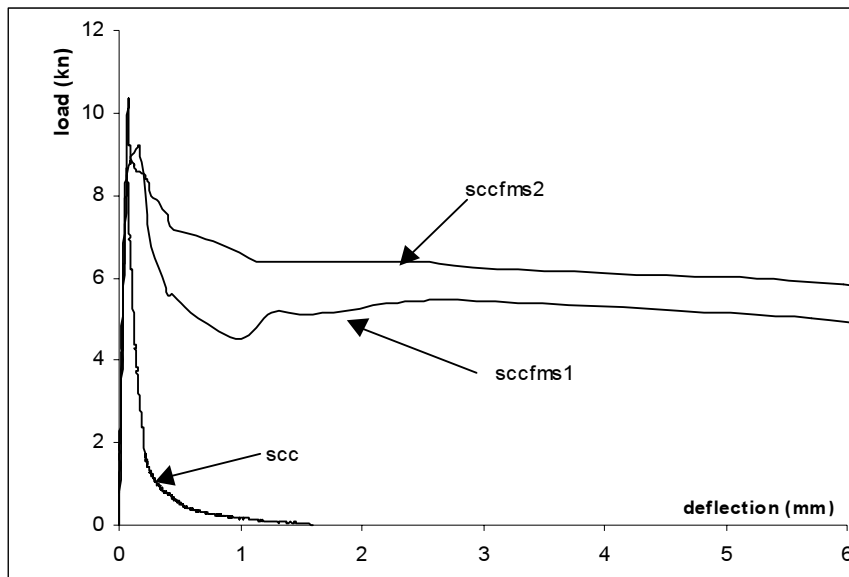


Fig.2: plots of flexural load versus deflection for hybrid fiber-reinforced self-compacting concrete beams

Table3: Total energy consumed and toughness index

| Type of concrete | σ_c' MPa | MOR (MPa) | Energy Kn.mm | Toughness index | | |
|------------------|--------------------|--------------|-----------------|-----------------|----------|----------|
| | | | E_{5mm} | I_5 | I_{10} | I_{30} |
| SCC | 36.03 | 4.01 | 2.30 | 3.06 | 3.85 | 4.55 |
| SCCMF1 | 38.5 | 5.11 | 18.10 | 2.50 | 4.50 | 6.57 |
| SCCMF2 | 38.36 | 5.44 | 26.06 | 2.92 | 3.78 | 14.56 |
| SCCSF | 36.4 | 4.8 | 38.56 | 3.50 | 8.88 | 17.73 |
| SCCFMS1 | 38.5 | 5.11 | 30.72 | 3.20 | 4.65 | 8.31 |
| SCCFMS2 | 41.4 | 6.15 | 39.04 | 4.96 | 10.74 | 18.80 |

relative humidity, and tested at 7 day age.

2-3 Testing procedures

For each mixture, nine specimens (six 100*100 mm cubes and three 100*100*500 mm beams) were prepared. The compressive and splitting tensile tests were carried out on the 100*100 mm cube specimens. The four-point loading flexural tests were carried out at a loading rate of 0.05 mm/mn on the 100*100*500 mm beams according to the requirements of ASTM C 1018 [11]. A notch 10 mm deep was cut at mid length of the prisms. During the flexural test, the load and the midspan deflection were recorded on a computerized data recording system, and the load –displacement curve was drawn.

3- Test results

All test results are summarized in Table 3, while graphical representations of the results are displayed in Figures1 and 2. Each strength value presented in Table 3 is the

average of three specimens. A total of 54 specimens were tested in this investigation.

3-1 Compressive strength

From Table 3, it was found that among the two types of fibers, amorphous steel fibers (30 mm) long gave the highest compressive strength, polypropylene fibers gave the lowest compressive strength. When the fibers were used in hybrid form, it obviously increased strength in the case of amorphous steel fibers (20mm) long - pp fibers (sccfms2). In the case of amorphous steel fibers (30mm) long - pp fibers, compressive slightly increased compared to simple amorphous steel and simple pp fibers.

3-2 Modulus of rupture (MOR)

Fiber addition increased MOR with all fibers. Among the two types of fibers, amorphous steel fibers gave the highest MOR. When the fibers were used in hybrid form, it increased MOR in the case of amorphous steel fibers (20mm) long - pp fibers compared to any of the simple fibers. In the case of amorphous

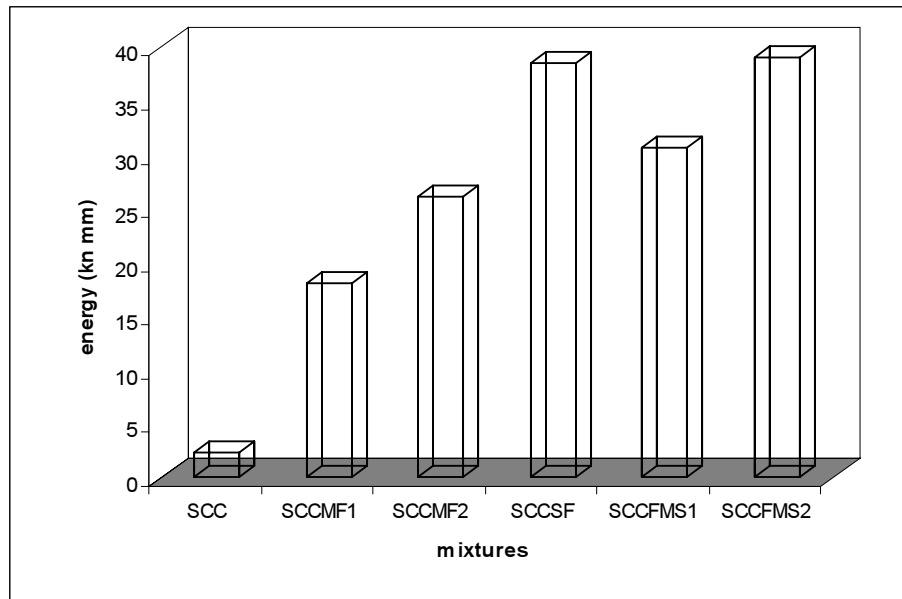


Fig.3 : The variation of rupture energies for different mixtures

steel fibers (30mm) long -pp fibers, MOR was slightly increased when compared to simple pp fibers and amorphous steel fibers.

3-3 Flexural toughness.

By bridging across macrocrack and reducing its opening, the fibers obviously affect the postpeak flexural softening response of the self-compacting concrete. Figure1 and 2 compare the load deflection of self-compacting concrete beams reinforced with amorphous steel fibers (20mm and 30 mm), pp fibers, and their two hybrid forms. From the figures, it was found that for the unreinforced self compacting concrete, the material demonstrated relative brittle behaviour, the load decreases rapidly with increase of midspan deflection after peak load. However, for the reinforced self compacting concretes, the decrease trends were flatter. Moreover the load platform or pseudohardening responses appeared generally, which were much dependent on the type of the added fibers and their combinations. A summary of calculated

toughness indices is reported in Table 3. The data showed that the toughness was increased with all fibers [12]. Among the three individual fibers, pp fibers gave the highest flexural toughness values for all of indice I5, I10 , and I30 , while steel (l=30mm) fibers gave the lowest toughness index of I30, and steel fibers (20mm) long gave the lowest toughness indices of I5 and I10. However, in hybrid form, the ductility characteristics were dramatically improved in case of pp-steel fibers (20mm) long. It gave the highest flexural toughness, which was much higher compared to that of simple pp fibers or steel fibers, especially for the index of I30, as shown in Figure.2. Steel fibers (20mm) long -pp fibers and steel fibers (30mm) long -pp fibers demonstrated similar flexural toughness when compared to simple pp fibers or steel fibers.

3-4 Energetic approach

For each composition energy was computed till a deflection of 5 mm (E5mm). This energy is an indicative of the mix toughness,

particularly in post-fissuration domain [4]. More it is important, more the cementitious composite is tenacious. The results are shown in Table3 and plotted in Figure3.

We notice first of all the fibers importance in changing the toughness of hybrid fibers self-compacting concrete. The highest energy value is observed for SCCFMS2 which contain exactly the same number of metallic fibers as SCCFMS1 but with different dimensions (the fibers in sccfms2 are shorter).

The toughness index during the loading have been also calculated for each type of concrete and the results are presented in Table3.

For each mix the dissipated energy until the first crack were calculated. We observe that toughness increases for all hybrids mix relatively to SCC without fiber. However SCCFMS1 gives the lowest toughness index relatively to other hybrids mix, but with a failure energy intermediate when compared to other mix. This is probably due to the length of metallic fibers (slenderness). It is proved that The short fibers were the most adapted in such cases. These short metallic fibers are randomly mixed in the cementous matrix with workability and remarkable setting that gave them a better uniform distribution [7]. The synergy (in term of toughness index) between different mix has been analysed and better performances are obtained with hybrid mix of short fibers as shown in Table 3.

4- Discussion and conclusion

In order to strengthen the matrix, the specific fiber spacing must be decreased to reduce the allowable flaw size [6]. This may be achieved by using short fibers, such as steel

fibers ($l=20\text{mm}$). These fibers can provide bridging of the micro-cracks before they reach the critical flaw size. To provide the toughening component, fibers of high ultimate strain capacity are required so that they can bridge the macrocracks in cementous matrix, and pp fibers or steel fibers are used for this purpose. Between these two fibers, pp was a low modulus fiber, the hybrid systems containing pp appeared to be less effective in controlling matrix crack opening. From the test results above, it was found that the main advantage of polypropylene fiber addition is the resulting high compressive and splitting tensile strengths, while the main advantage of steel fiber addition is the resulting high MOR and flexural toughness. Therefore, the pp-steel fiber($l=20\text{mm}$) hybrid was the most beneficial for the improvement of strength and flexural toughness. A clear synergy was observed in the hybrid self-compacting concrete containing steel fibers ($l=20\text{mm}$) and pp fibers. From Table 3, improvement of 15% in compressive strength, 53% in MOR, and 62_313% in toughness indices were obtained for pp-amorphous steel fiber ($l=20\text{mm}$) hybrid composite compared to unreinforced self compacting concrete.

As can be seen in Figure2, the load carrying capacity for the amorphous steel fiber (30mm) long -pp hybrid decreases rapidly in the postpeak region. The brittle response may be attributed to the length of fibers. However, the decrease in load was recovered as the steel fibers began to pull out from matrix. The toughness of amorphous steel fibers ($l=20\text{mm}$)- pp hybrids represented an increase of about 186% and 6% for I30 compared to simple steel fibers (30mm) long and pp fibers, respectively.

Obviously, the presence of pp fibers had increased the resistance of the composite

reinforced with randomly distributed short steel fibers and vice versa. As a result, the potential strength capacities of pp fibers or steel fibers were better utilized and the strength and flexural toughness of steel fibers (20mm) long - pp hybrids were hence higher than the all other composites.

Various conclusions can be drawn from this experimental study. The test results first indicated that at same number of fiber 30mm long and 20mm long, it is possible to obtain with 20mm long, material with the enhanced strength and improved toughness from hybrid fibers.

While the addition of fibers does not enhance the compressive strength of the mix, but the hybrids form clearly enhances the MOR.

Hybrids based on steel fibers (20mm) long and polypropylene fibers demonstrated some synergy.

Of the two amorphous steel fibers investigated, the short fiber is clearly far more effective in producing synergy in hybrids than the length fiber.

While hybrid fiber reinforced self compacting concrete are promising, and have been used in several areas [13], there is much further research needed to develop the science and rationale necessary for their optimization.

Further tests are carried out for the case of polypropylene fibers length and will be the object of futur paper in the same journal.

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